## PREDEVELOPMENT vs. POSTDEVELOPMENT RUNOFF VOLUME ANALYSIS:

A Simple Real-World Example To Demonstrate The Methodology

PRESENTED AT THE FES<br>STORMWATER MANAGEMENT DESIGNER'S COURSE APRIL 2, 1998

## presented by:

Devo Seereeram, Ph.D., P.E.
Consulting Geotechnical Engineer
www.iag.net/~devo

## PREDEVELOPMENT



## POSTDEVELOPMENT - DRY BOTTOM POND



## POSTDEVELOPMENT - WET BOTTOM POND



## EXAMPLE PROBLEM MODEL INPUT: DRAINAGE BASIN PARAMETERS

|  | PARAMETER | MAGNITUDE |  |
| :--- | :---: | :---: | :---: |
|  |  |  | POST |
| Area of contributing drainage basin | $\mathrm{ft}^{2}$ | 35,031 | 35,031 |
| Area of contributing drainage basin | acre $^{2}$ | 0.804 | 0.804 |
| Non-impervious (non-DCIA) area | $\mathrm{ft}^{2}$ | 35,031 | 12,296 |
| Curve Number (CN) for non-DCIA area (AMC I) | - | 30 | 30 |
| Curve Number (CN) for non-DCIA area (AMC II) |  | 49 | 49 |
| Curve Number (CN) for non-DCIA area (AMC III) |  | 69 | 69 |
| Impervious area (DCIA) | $\mathrm{ft}^{2}$ | 0 | 22,735 |
| Directly connected impervious area | $\%$ | $0.00 \%$ | $64.90 \%$ |

## EXAMPLE PROBLEM MODEL INPUT: RAINFALL DATA

| Month | Average for Orlando Intl Airport [1964-93] (inch) | Normal Year [1982] (inch) |
| :---: | :---: | :---: |
| January | 2.23 | 1.72 |
| February | 2.70 | 1.34 |
| March | 3.53 | 4.85 |
| April | 2.62 | 6.27 |
| May | 3.40 | 5.29 |
| June | 6.98 | 6.06 |
| July | 7.83 | 11.81 |
| August | 6.68 | 5.03 |
| September | 6.74 | 6.96 |
| October | 3.36 | 0.74 |
| November | 1.88 | 0.53 |
| December | 1.99 | 1.01 |
| TOTALS | 49.94 | 51.61 |

Each simulation was run for a 365-day (1 year) period starting January 1, 1982 and ending December 31, 1982. The simulation time step was 24 hours (i.e., 1 day). The total rainfall over each $24-\mathrm{hr}$ period was treated as a single rainfall event for computing stormwater runoff.

## EXAMPLE PROBLEM MODEL INPUT: STAGE-AREA DATA OF POND

| Geometric Parameters for Pond |  |  |  |
| :--- | :---: | :---: | :---: |
| Parameter | Unit | Magnitude |  |
| Equivalent pond length | ft | 120 |  |
| Equivalent pond width | ft | $\mathbf{2 5}$ |  |


| Stage <br> $(\mathrm{ft} \mathrm{NGVD})$ | Area <br> Excluding Soil Voids in Side Slope <br> $\left(\mathrm{ft}^{2}\right)$ | Area <br> Including Soil Voids in Side Slope <br> $\left(\mathrm{ft}^{2}\right)$ |
| :---: | :---: | :---: |
| 101.0 | 0 | 548 |
| 104.0 | 756 | 1,152 |
| 105.0 | 1,484 | 1,735 |
| 106.0 | 2,738 | 2,738 |
| 107.0 | 3,876 | 3,876 |

## EXAMPLE PROBLEM MODEL INPUT: POND DISCHARGE STURCTURES

| Description | Parameter | Unit | Magnitude |
| :--- | :--- | :---: | :---: |
| SIDE CONTROL <br> WEIR | Discharge elevation | ft NGVD | 106.09 |
|  | Weir length | ft | 1.83 |
|  | Weir coefficient | - | 3.13 |
|  | Weir exponent | - | 1.5 |
| TOP <br> WEIR | ft NGVD | 106.22 |  |
|  | Discharge elevation | ft | 7.83 |
|  | Weir length | - | 3.13 |
|  | Weir coefficient | - | 1.5 |
|  | Weir exponent | ft NGVD | 106.90 |
| DROP CURB TO <br> ENTR <br> BLVD | Discharge elevation | ft | 30 |
|  | Weir length | - | 2.861 |
|  | Weir coefficient | - | 1.5 |
|  | Weir exponent |  |  |

## EXAMPLE PROBLEM MODEL INPUT: AQUIFER PARAMETERS

| Parameter | Unit | Magnitude |
| :--- | :---: | :---: |
| Base of mobilized aquifer | ft NGVD | +99 |
| Seasonal high water table | ft NGVD | +101 |
| Horizontal hydraulic conductivity | ft/day | 5 |
| Fillable porosity | $\%$ | 20 |
| Vertical recharge to Floridan aquifer within pond | in/yr | 6 |
| Vertical recharge to Floridan aquifer outside pond | in/yr | 6 |

## EXAMPLE PROBLEM MODEL INPUT: EVAPORATION \& EVAPOTRANSPIRATION

| Month | No. of days in month | Monthly rates (inch) |  |  | Daily rates (inch) |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Normal Rainfall | Lake Evaporation | Evapotranspiration | Lake Evaporation | Evapotranspiration |
| January | 31 | 2.10 | 2.2 | 1.969 | 0.07097 | 0.06350 |
| February | 28 | 2.83 | 2.5 | 1.850 | 0.08929 | 0.06609 |
| March | 31 | 3.20 | 3.9 | 2.677 | 0.12581 | 0.08636 |
| April | 30 | 2.19 | 5.5 | 3.307 | 0.18333 | 0.11024 |
| May | 31 | 3.96 | 6.7 | 3.071 | 0.21613 | 0.09906 |
| June | 30 | 7.39 | 5.8 | 4.882 | 0.19333 | 0.16273 |
| July | 31 | 7.78 | 6.2 | 4.764 | 0.20000 | 0.15367 |
| August | 31 | 6.32 | 5.5 | 4.449 | 0.17742 | 0.14351 |
| September | 30 | 5.62 | 4.4 | 4.094 | 0.14667 | 0.13648 |
| October | 31 | 2.82 | 3.6 | 4.055 | 0.11613 | 0.13081 |
| November | 30 | 1.78 | 2.4 | 2.323 | 0.08000 | 0.07743 |
| December | 31 | 1.83 | 2.2 | 1.969 | 0.07097 | 0.06350 |
| TOTALS | 365 | 47.82 | 50.9 | 39.409 |  |  |




Ready
-

## RESULTS FOR THIS EXAMPLE

## AVERAGE RAINFALL YEAR IS 1982

- Predevelopment runoff volume for calendar year 1982 is 4,012 cubic feet
- Postdevelopment runoff volume for calendar year 1982 is 16,171 cubic feet
- If soil permeability is doubled from 5 ft/day to 10 ft/day, the postdevelopment runoff volume for calendar year 1982 is 5,676 cubic feet
- If soil permeability is quadrupled from 5 ft/day to 20 ft/day, the postdevelopment runoff volume for calendar year 1982 is 1,563 cubic feet $(16,171 \rightarrow 5,676 \rightarrow 1,563)$

